

TITAN PLATE C CONCRETE



PLATES FOR SHEAR LOADS

VERSATILE

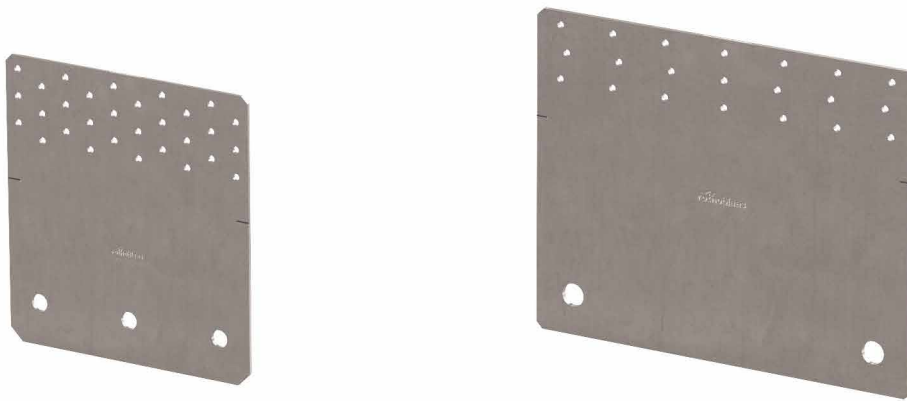
Suitable for a continuous fastening of both CLT (Cross Laminated Timber) panels and framed panels to the sub-structure.

INNOVATIVE

Designed to be partially or completely fastened with nails or screws. Possibility of installation even in the presence of bedding mortar.

CALCULATED AND CERTIFIED

CE marking according to EN 14545. Available in 2 versions. TCP300 with increased thickness optimised for CLT.



CHARACTERISTICS

FOCUS	shear joints on concrete
HEIGHT	200 300 mm
THICKNESS	3.0 4.0 mm
FASTENERS	LBA, LBS, VIN-FIX, HYB-FIX, AB1, SKR



MATERIAL

Bright zinc plated carbon steel, two dimensional perforated plate.

FIELDS OF USE

Timber-to-concrete shear joints for panels and timber beams

- CLT, LVL
- solid timber and glulam
- framed structures (platform frame)
- timber based panels



ADDED STOREYS




Ideal for making flat joints between concrete or masonry elements and CLT panels. Construction of continuous shear connections.

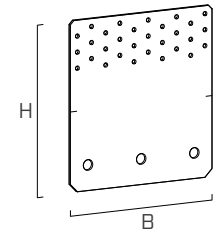
CONCRETE KERB

Versatile fastening configurations. Solutions designed, calculated, tested and certified with partial and total fastening, with horizontal or vertical fibre direction.

CODES AND DIMENSIONS

TITAN PLATE TCP

CODE	B [mm]	H [mm]	holes	$n_v \varnothing 5$ [pcs]	s [mm]		pcs
TCP200	200	214	Ø13	30	3		10
TCP300	300	240	Ø17	21	4		5



MATERIAL AND DURABILITY

TCP200: carbon steel DX51D+Z275.

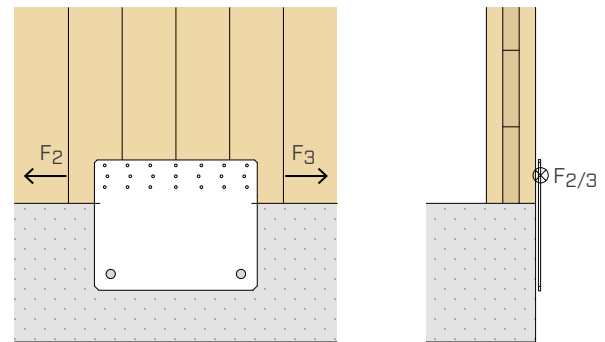
TCP300: S355 bright zinc plated carbon steel.

To be used in service classes 1 and 2 (EN 1995-1-1).

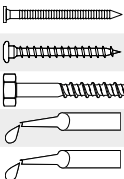
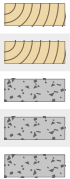


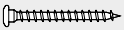

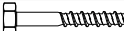



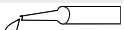

FIELD OF USE

- Timber to concrete joints

EXTERNAL LOADS



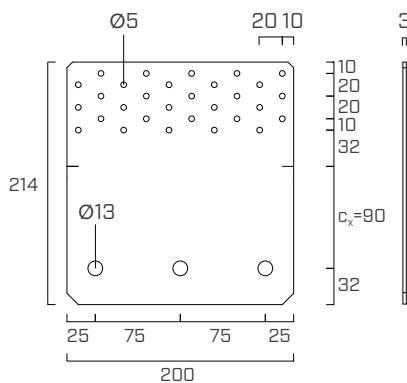
ADDITIONAL PRODUCTS - FASTENING

type	description		d [mm]	support 
LBA	Anker nail		4	
LBS	screw for plates		5	
SKR	screw anchor		12 - 16	
VIN-FIX ^(*)	chemical anchor		M12 - M16	
HYB-FIX	chemical anchor		M12 - M16	

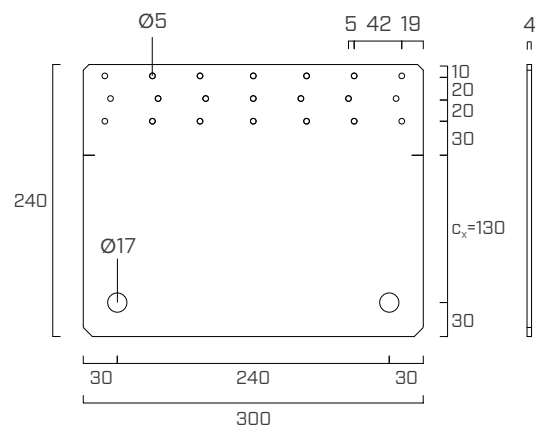
^(*) For more information, see the data sheet available at www.rothoblaas.com

GEOMETRY

TCP200



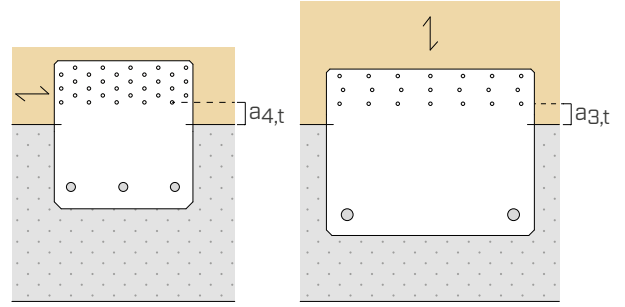
TCP300



INSTALLATION

TIMBER minimum distances		nails LBA Ø4	screws LBS Ø5
C/GL	$a_{4,t}$ [mm]	≥ 20	≥ 25
CLT	$a_{3,t}$ [mm]	≥ 28	≥ 30

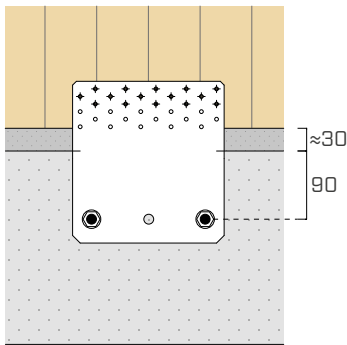
- C/GL: minimum distances for solid timber or glulam consistent with EN 1995-1-1 according to ETA considering a timber density $\rho_k \leq 420 \text{ kg/m}^3$
- CLT minimum distances for Cross Laminated Timber according to ÖNORM EN 1995-1-1 (Annex K) for nails and ETA 11/0030 for screws



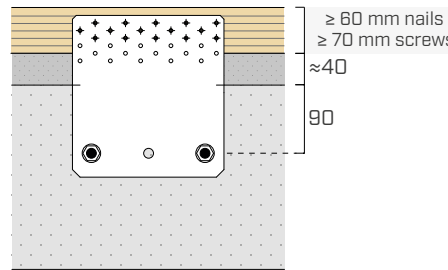
PARTIAL FASTENING

In the presence of design requirements such as varying stress values or the presence of a levelling layer between the wall and the support surface, it is possible to use pre-calculated **partial nailing** or to position the plates as required (e.g. lowered plates) taking care to respect the minimum distances indicated in the table and verify the strength of the anchor-to-concrete group taking into account the increase in distance from the edge (c_x). Below there are some examples of possible limit configurations:

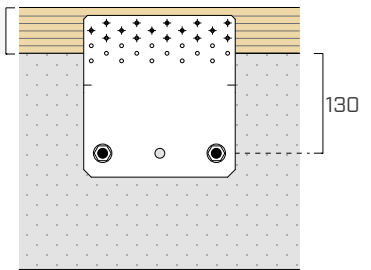
TCP200



PARTIAL 15 FASTENINGS - CLT

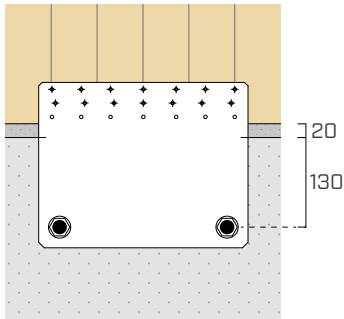


PARTIAL 15 FASTENINGS - C/GL

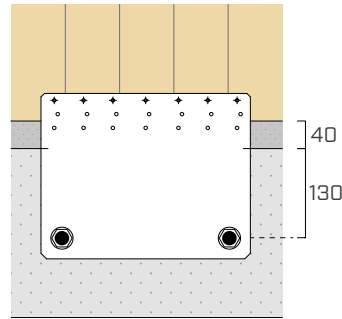


LOWERED PLATE - C/GL

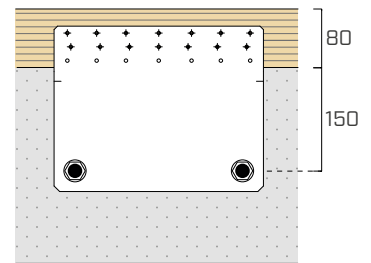
TCP300



PARTIAL 14 FASTENINGS - CLT

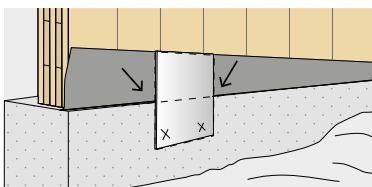


PARTIAL 7 FASTENINGS - CLT

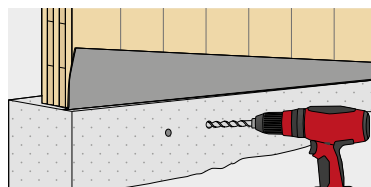


LOWERED PLATE - C/GL

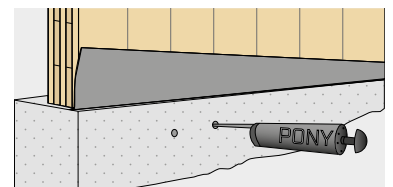
ASSEMBLY



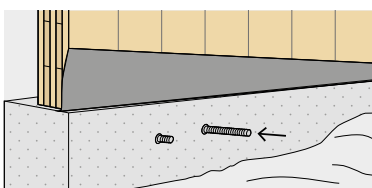
Positioning of the TITAN TCP with the dashed line at the timber-concrete interface and hole marking



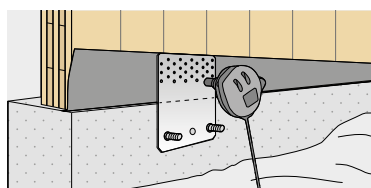
Removal of the TITAN TCP plate and drilling of the concrete support



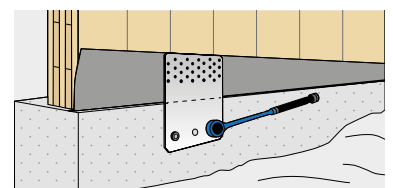
Accurate hole cleaning



Injection of the anchor and insertion of the threaded rods into the holes



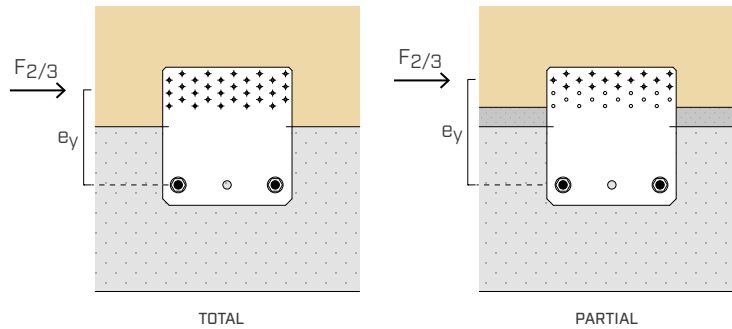
Installation of the TITAN TCP and nailing



Positioning of nuts and washers by adequate tightening

STRUCTURAL VALUES | SHEAR JOINT | TIMBER-TO-CONCRETE

TCP200



TIMBER STRENGTH

configuration on timber	TIMBER					STEEL		CONCRETE		
	holes fastening Ø5	$R_{2/3,k}$ timber ⁽¹⁾	$R_{2/3,k}$ CLT ⁽²⁾	$R_{2/3,k}$ steel	γ_{steel}	holes fastening Ø13	e_y ⁽³⁾			
	type	Ø x L [mm]	n_v [pcs]	[kN]	[kN]	[kN]	γ_{M2}	Ø [mm]	n_v [pcs]	[mm]
• total fastening	LBA nails	Ø4,0 x 60	30	55,6	70,8	21,8	γ_{M2}	M12	2	147
	screws LBS	Ø5,0 x 60	30	54,1	69,9					
• partial fastening	LBA nails	Ø4,0 x 60	15	27,8	35,4	20,5	γ_{M2}	M12	2	162
	screws LBS	Ø5,0 x 60	15	27,0	35,0					

CONCRETE STRENGTH

Concrete strength values of some of the possible anchoring solutions, according to the configurations adopted for fastening on timber (e_y). It is assumed that the plate is positioned with the assembly notches at the timber-to-concrete interface (distance between anchor and concrete edge $c_x = 90$ mm).

configuration on concrete	holes fastening Ø13		total fastening ($e_y = 147$ mm)	partial fastening ($e_y = 162$ mm)
	type	Ø x L [mm]	$R_{2/3,d}$ concrete [kN]	$R_{2/3,d}$ concrete [kN]
• uncracked	VIN-FIX 5.8	M12 x 140	12,6	11,5
		M12 x 195	13,4	12,2
	SKR-CE	12 x 90	12,6	11,4
	AB1	M12 x 100	13,1	11,9
• cracked	VIN-FIX 5.8	M12 x 140	8,9	8,1
		M12 x 195	9,5	8,7
	SKR-CE	12 x 90	8,9	8,1
	AB1	M12 x 100	9,2	8,4
• seismic	HYB-FIX 8.8	M12 x 140	6,6	6,1
		M12 x 195	8,1	7,4

NOTES:

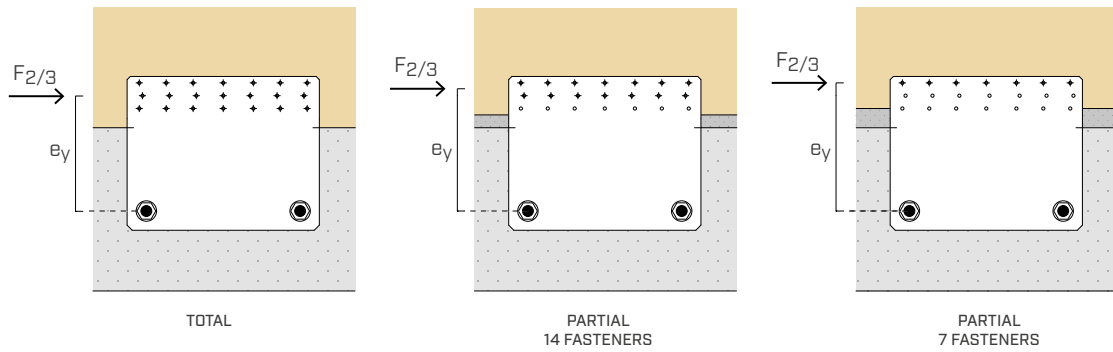
⁽¹⁾ Strength values for use on solid timber or glulam platform beam, calculated considering the effective number according to Table 8.1 (EN 1995 -1-1).

⁽²⁾ Strength values for use on CLT.

⁽³⁾ Eccentricity of calculation for verification of the anchor-to-concrete group.

STRUCTURAL VALUES | SHEAR JOINT | TIMBER-TO-CONCRETE

TCP300



TIMBER STRENGTH

configuration on timber	TIMBER					STEEL		CONCRETE						
	holes fastening Ø5			$R_{2/3,k \text{ timber}}^{(1)}$	$R_{2/3,k \text{ CLT}}^{(2)}$	$R_{2/3,k \text{ steel}}$		holes fastening Ø17		$e_y^{(3)}$ [mm]				
	type	Ø x L [mm]	n_v [pcs]	[kN]	[kN]	[kN]	γ_{steel}	Ø [mm]	n_v [pcs]					
• total fastening	LBA nails	Ø4,0 x 60	21	38,4	49,6	64,0	γ_{M2}	M16	2	180				
	screws LBS	Ø5,0 x 60	21	36,9	48,9									
• partial fastening 14 fasteners	LBA nails	Ø4,0 x 60	14	25,6	33,0	60,5	γ_{M2}			M16	2	190		
	screws LBS	Ø5,0 x 60	14	24,6	32,6									
• partial fastening 7 fasteners	LBA nails	Ø4,0 x 60	7	12,8	16,5	57,6	γ_{M2}					M16	2	200
	screws LBS	Ø5,0 x 60	7	12,3	16,3									

CONCRETE STRENGTH

Concrete strength values of some of the possible anchoring solutions, according to the configurations adopted for fastening on timber (e_y). It is assumed that the plate is positioned with the assembly notches at the timber-to-concrete interface (distance between anchor and concrete edge $c_x = 130$ mm).

configuration on concrete	holes fastening Ø17		total fastening ($e_y = 180$ mm)	partial fastening ($e_y = 190$ mm)	partial fastening ($e_y = 200$ mm)
	type	Ø x L [mm]	$R_{2/3,d \text{ concrete}}$		
			[kN]	[kN]	[kN]
• uncracked	VIN-FIX 5.8	M16 x 195	29,6	28,3	27,0
	SKR-CE	16 x 130	29,7	28,2	26,8
	AB1	M16 x 145	30,2	28,7	27,3
• cracked	VIN-FIX 5.8	M16 x 195	21,0	20,0	19,1
	SKR-CE	16 x 130	21,0	19,9	19,0
	AB1	M16 x 145	21,4	20,3	19,3
• seismic	HYB-FIX 8.8	M16 x 195	16,8	16,2	15,6
		M16 x 245	18,6	17,7	16,9

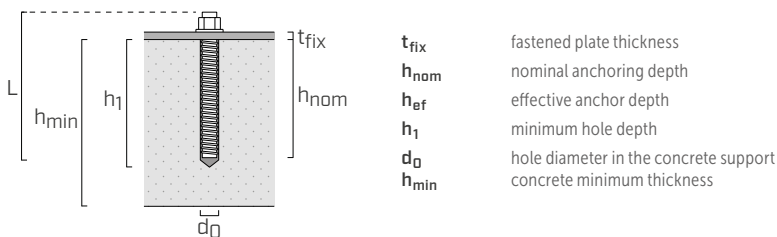
GENERAL PRINCIPLES:

General calculation principles see page 7

ANCHORS INSTALLATION PARAMETERS | TCP200 - TCP300

installation	anchor type		t_{fix} [mm]	h_{ef} [mm]	h_{nom} [mm]	h_1 [mm]	d_0 [mm]	h_{min} [mm]
	type	$\varnothing \times L$ [mm]						
TCP200	VIN-FIX 5.8	M12 x 140	3	112	112	120	14	150
	HYB-FIX 8.8							
	SKR-CE	12 x 90	3	64	87	110	10	
	AB1	M12 x 100	3	70	80	85	12	
	VIN-FIX 5.8	M12 x 195	3	170	170	175	14	200
HYB-FIX 8.8								
TCP300	VIN-FIX 5.8	M16 x 195	4	164	164	170	18	200
	HYB-FIX 8.8							
	SKR-CE	16 x 130	4	85	126	150	14	
	AB1	M16 x 145	4	85	97	105	16	
	HYB-FIX 8.8	M16 x 245	4	210	210	215	18	250

INA precut threaded rod complete with nut and washer: see INA data sheet at www.rothoblaas.com



ANCHORS FOR CONCRETE VERIFICATION | TCP200 - TCP300

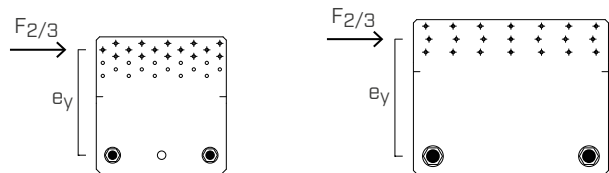
Fastening to concrete using anchors must be verified on the basis of the stressing forces of the anchors, which depend on the timber fastening configuration.

The position and number of nails/screws determine the e_y eccentricity value, understood as the distance between the centre of gravity of the nailing and that of the anchors.

The anchor group must be verified for:

$$V_{Sd,x} = F_{2/3,d}$$

$$M_{Sd,z} = F_{2/3,d} \times e_y$$



GENERAL PRINCIPLES:

- Characteristic values according to EN 1995-1-1. The design values of the anchors for concrete are calculated in accordance with the respective European Technical Assessments.

The connection design strength value is obtained from the values on the table as follows:

$$R_d = \min \left\{ \begin{array}{l} \frac{(R_{k, \text{timber}} \text{ or } R_{k, \text{CLT}}) \cdot k_{mod}}{Y_M} \\ \frac{R_{k, \text{steel}}}{Y_{steel}} \\ R_{d, \text{concrete}} \end{array} \right.$$

The coefficients k_{mod} , Y_M and Y_{steel} should be taken according to the current regulations used for the calculation.

- The calculation process used a timber characteristic density of $\rho_k = 350 \text{ kg/m}^3$ and C25/30 concrete with a thin reinforcing layer and minimum thickness indicated in the table.
- Dimensioning and verification of timber and concrete elements must be carried out separately.
- The strength values are valid for the calculation hypotheses defined in the table; for boundary conditions different from the ones in the table (e.g. minimum distances from the edge), the anchors-to-concrete can be verified using MyProject calculation software according to the design requirements.
- Seismic design in performance category C2, without ductility requirements on anchors (option a2) elastic design according to EOTA TR045. For chemical anchors it is assumed that the annular space between the anchor and the plate hole is filled ($a_{gap} = 1$).

EXPERIMENTAL INVESTIGATIONS | TCP300

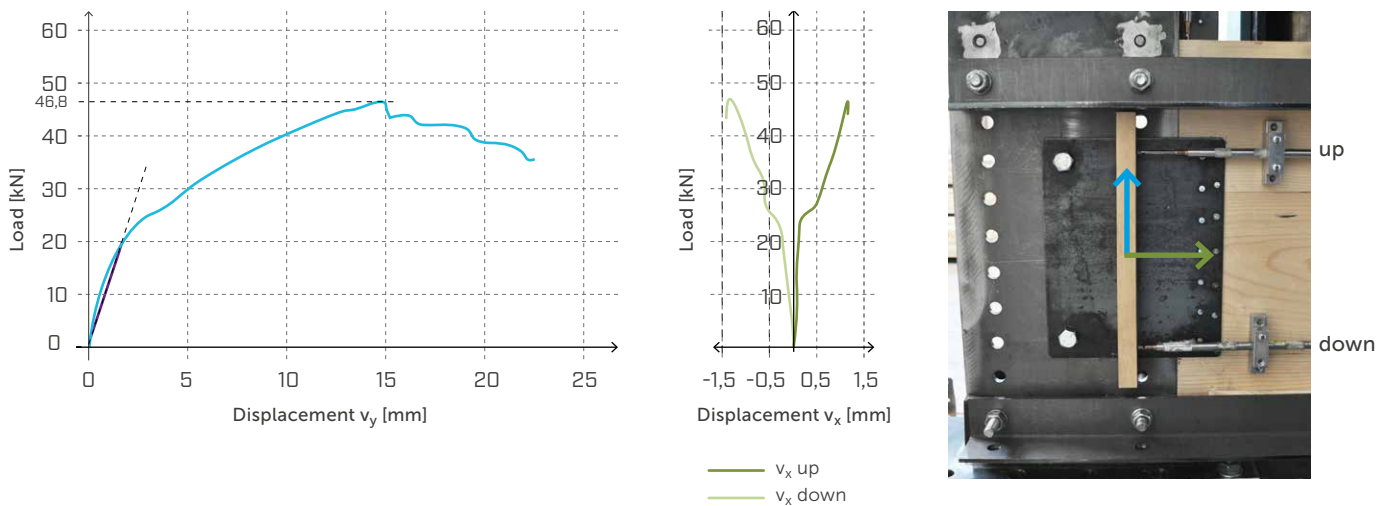
In order to calibrate the numerical models used for the design and verification of the TCP300 plate, an experimental campaign was carried out in collaboration with the Institute for BioEconomy (IBE) - San Michele all'Adige.

The connection system, nailed or screwed to CLT panels, has been shear stressed through monotonic tests in displacement control registering the load, displacement in the two main directions and collapse mode.

The results obtained were used to validate the analytical calculation model for the TCP300 plate, based on the hypothesis that the shear centre is placed at the centre of gravity of the fastenings on timber and therefore that the anchors, usually the weak point of the system, are stressed not only by the shear actions but also by the local moment.

The study in different fastening configurations (Ø4 nails/Ø5 screws, full nailing, partial nailing with 14 connectors, partial nailing with 7 connectors) shows that the mechanical behaviour of the plate is strongly influenced by the **relative stiffness of the connectors on timber** compared to that of the anchors, in tests simulated by bolting on steel.

In all cases a shear failure mode of the timber fasteners has been observed that does not result in evident plate rotation. Only in some cases (full nailing) the non-negligible rotation of the plate leads to an increase in stress on the timber fasteners resulting from a redistribution of the local moment with consequent stress relief on the anchors, which represent the limiting point of the overall strength of the system.



Load-to-displacement diagrams for TCP300 specimen with partial nailing (no. 14 LBA Ø4 x 60 mm nails).

Further investigations are necessary in order to define an analytical model that can be generalized to the different configurations of use of the plate that is able to provide the actual stiffness of the system and the redistribution of stresses as the boundary conditions (connectors and base materials) vary.